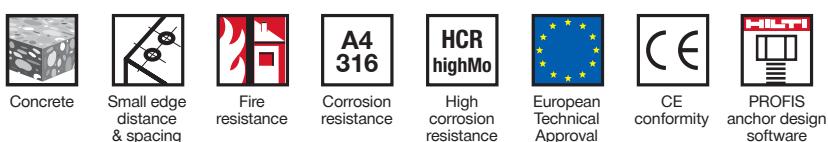


HVU with HAS-E rod adhesive anchor

Mortar System	Benefits
 Hilti HVU foil capsule HAS rods HAS-E (Zinc) HAS-E-F (Gal) HAS-E-R (A4-70) HAS-HCR rods	<ul style="list-style-type: none"> ■ suitable for non-cracked concrete C 20/25 to C 50/60 ■ high loading capacity ■ suitable for dry and water saturated concrete ■ large diameter applications ■ high corrosion resistant



Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval ^{a)}	DIBt, Berlin	ETA-05/0255 / 2011-06-23
Fire test report	IBMB, Braunschweig	UB-3333/0891-1 / 2004-03-26
Fire test report ZTV-Tunnel	IBMB, Braunschweig	UB 3333/0891-2 / 2003-08-12
Assessment report (fire)	warringtonfire	WF 166402 / 2007-10-26

a) All data given in this section according ETA-05/0255, issue 2011-06-23

Service temperature range

Hilti HVU adhesive may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +40 °C	+24 °C	+40 °C
Temperature range II	-40 °C to +80 °C	+50 °C	+80 °C
Temperature range III	-40 °C to +120 °C	+72 °C	+120 °C

Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

Design process for typical anchor layouts

The design values in the tables are obtained from the design method according to ETAG 001, Annex C and Hilti simplified design method. Design resistance according to data given in ETA-05/0255, issue 2011-06-23.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

The values are valid for the anchor configuration.

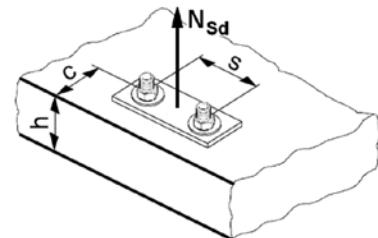
For more complex fastening applications please use the anchor design software PROFIS Anchor.

STEP 1: TENSION LOADING

The design tensile resistance N_{Rd} is the lower of:

- Combined pull-out and concrete cone resistance

$$N_{Rd,p} = f_{B,p} \cdot N^{*}_{Rd,p}$$



$N^{*}_{Rd,p}$ is obtained from the relevant design tables

$f_{B,p}$ influence of concrete strength on combined pull-out and concrete cone resistance

Concrete Strengths $f'_{c,cyl}$ (MPa)	20	25	32	40	50
$f_{B,p}$	0.95	0.97	1.00	1.02	1.04

- Concrete cone or concrete splitting resistance

$$N_{Rd,c} = f_B \cdot N^{*}_{Rd,c}$$

$N^{*}_{Rd,c}$ is obtained from the relevant design tables

f_B influence of concrete strength on concrete cone resistance

Concrete Strengths $f'_{c,cyl}$ (MPa)	20	25	32	40	50
f_B	0.79	0.87	1.00	1.11	1.22

- Design steel resistance (tension) $N_{Rd,s}$

Anchor size	M8	M10	M12	M16	M20	M24
$N_{Rd,s}$ HAS - E 5.8 [kN]	11.3	17.3	25.3	48.0	74.7	106.7
HAS-E-R [kN]	12.3	19.8	28.3	54.0	84.0	119.8

$$N_{Rd} = \min \{ N_{Rd,p}, N_{Rd,c}, N_{Rd,s} \}$$

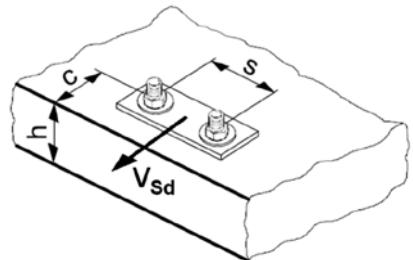
CHECK $N_{Rd} \geq N_{sd}$

STEP 2: SHEAR LOADING

The design shear resistance V_{Rd} is the lower of:

■ Design Concrete Edge Resistance

$$V_{Rd,c} = f_B \cdot V^{*}_{Rd,c}$$



$V^{*}_{Rd,c}$ is obtained from the relevant design table

f_B influence of concrete strength

Concrete Strengths f'_c, cyl (MPa)	20	25	32	40	50
f_B	0.79	0.87	1.00	1.11	1.22

Shear load acting parallel to edge:

These tables are for a single free edge only

2 anchors:

For shear loads acting parallel to this edge, the concrete resistance $V^{*}_{Rd,c}$ can be multiplied by the factor = 2.5

4 anchors:

For shear loads acting parallel to the edge - the anchor row closest to the edge is checked to resist half the total design load. To obtain the concrete resistance use the corresponding 2 anchor configuration $V^{*}_{Rd,c}$ and multiply by the factor = 2.5

■ Design steel resistance (shear): $V_{Rd,s}$

Anchor size	M8	M10	M12	M16	M20	M24
$V_{Rd,s}$ HAS – E 5.8 [kN]	6.8	10.4	15.2	28.8	44.8	64.0
HAS-E-R [kN]	7.7	11.5	17.3	32.7	50.6	71.8

$$V_{Rd} = \min \{ V_{Rd,c}, V_{Rd,s} \}$$

CHECK $V_{Rd} \geq V_{sd}$

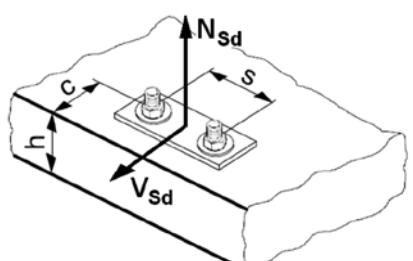
STEP 3: COMBINED TENSION AND SHEAR LOADING

The following equations must be satisfied:

$$N_{sd}/N_{Rd} + V_{sd}/V_{Rd} \leq 1.2$$

and

$$N_{sd}/N_{Rd} \leq 1, V_{sd}/V_{Rd} \leq 1$$



Precalculated table values – design resistance values

General:

The following tables provide the total ultimate limit state design resistance for the configurations.
All tables are based upon:

- correct setting (See setting instruction)
- non-cracked concrete – $f_{c,cyl} = 32 \text{ MPa}$
- temperature range I (see service temperature range)
- base material thickness, as specified in the table
- One typical embedment depth, as specified in the tables

Basic loading data (for a single anchor) – no edge distance and spacing influence

Embedment depth and base material thickness for the basic loading data

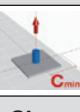
Anchor size	M8	M10	M12	M16	M20	M24
Typical embedment depth h_{ef} [mm]	80	90	110	125	170	210
Base material thickness h [mm]	110	120	140	170	220	270

Design resistance [kN] – uncracked concrete, 32 Mpa

Anchor size	M8	M10	M12	M16	M20	M24
Non-cracked concrete						
Tensile	Pull-out $N^*_{Rd,p}$	17.9	25.0	35.8	42.9	82.3
	Concrete $N^*_{Rd,c}$	30.5	36.4	49.1	59.5	129.6
Shear	$V_{Rd,s}$	Steel governed refer $V_{Rd,s}$ table				

Basic loading data (for a single anchor) – with minimum edge distance

Design resistance [kN] - uncracked concrete, 32 Mpa

Anchor size	M8	M10	M12	M16	M20	M24
Min. edge distance c_{min} [mm]	40	45	55	65	90	120
Min Base thickness h_{min} [mm]	110	120	140	170	220	270
Tensile N_{Rd}						
	Pull-out $N^*_{Rd,p} [\text{kN}]$	10.1	13.6	19.7	23.6	46.0
	Concrete $N^*_{Rd,c} [\text{kN}]$	14.5	17.1	23.0	28.5	63.0
Shear V_{Rd}						
	Shear $V^*_{Rd,c} [\text{kN}]$ (without lever arm)	4.7	5.9	8.3	11.2	19.1
						29.8

Materials

Mechanical properties of HAS

		Data according ETA-05/0255, issue 2011-06-23						
Anchor size		M8	M10	M12	M16	M20	M24	M30
Nominal tensile strength f_{uk}	HAS-(E) 5.8 [N/mm ²]	500	500	500	500	500	500	-
	HAS-(E)(F) 8.8 [N/mm ²]	800	800	800	800	800	800	800
	HAS -(E)R [N/mm ²]	700	700	700	700	700	700	500
	HAS -(E)HCR [N/mm ²]	800	800	800	800	800	700	-
Yield strength f_{yk}	HAS-(E) [N/mm ²]	400	400	400	400	400	400	-
	HAS-(E)(F) 8.8 [N/mm ²]	640	640	640	640	640	640	640
	HAS -(E)R [N/mm ²]	450	450	450	450	450	450	210
	HAS -(E)HCR [N/mm ²]	640	640	640	640	640	400	-
Stressed cross-section A_s	HAS [mm ²]	32.8	52.3	76.2	144	225	324	519
Section modulus Z	HAS [mm ³]	27.0	54.1	93.8	244	474	809	1706
Steel failure with lever arm		M8	M10	M12	M16	M20	M24	M30
Design bending moment $M_{Rd,s}$	HAS-E-5.8 [Nm]	13	26	45	118	227	389	NA
	HAS-E-8.8 [Nm]	NA	NA	NA	NA	NA	NA	1310
	HAS-E-R [Nm]	15	29	51	131	255	436	430
	HAS-E-HCR [Nm]	21	42	72	187	364	389	819

Material quality

Part	Material
Threaded rod HAS-(E) M8-M24 HAS-(E) M27+M30	Strength class 5.8, EN ISO 898-1, $A_s > 8\%$ ductile steel galvanized $\geq 5 \mu\text{m}$, EN ISO 4042 (F) hot dipped galvanized $\geq 45 \mu\text{m}$, EN ISO 10684
Threaded rod HAS-(E)F M8-M30 HAS-(E) M27+M30	Strength class 8.8, EN ISO 898-1, $A_s > 8\%$ ductile steel galvanized $\geq 5 \mu\text{m}$, EN ISO 4042 (F) hot dipped galvanized $\geq 45 \mu\text{m}$, EN ISO 10684
Threaded rod HAS-(E)R	Stainless steel grade A4, $A_s > 8\%$ ductile strength class 70 for $\leq M24$ and class 50 for M27 to M30, EN ISO 3506-1, EN 10088: 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
Threaded rod HAS-(E)HCR	High corrosion resistant steel, EN ISO 3506-1, EN 10088: 1.4529; 1.4565 strength $\leq M20$: $R_m = 800 \text{ N/mm}^2$, $R_{p,0.2} = 640 \text{ N/mm}^2$, $A_s > 8\%$ ductile M24: $R_m = 700 \text{ N/mm}^2$, $R_{p,0.2} = 400 \text{ N/mm}^2$, $A_s > 8\%$ ductile
Washer ISO 7089	Steel galvanized, EN ISO 4042; hot dipped galvanized, EN ISO 10684
	Stainless steel, EN 10088: 1.4401
	High corrosion resistant steel, EN 10088: 1.4529; 1.4565
Nut EN ISO 4032	Strength class 8, ISO 898-2 steel galvanized $\geq 5 \mu\text{m}$, EN ISO 4042 hot dipped galvanized $\geq 45 \mu\text{m}$, EN ISO 10684
	Strength class 70, EN ISO 3506-2, stainless steel grade A4, EN 10088: 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
	Strength class 70, EN ISO 3506-2, high corrosion resistant steel, EN 10088: 1.4529; 1.4565

Anchor dimensions

Anchor size	M8	M10	M12	M16	M20	M24	M30 a)
Anchor rod HAS-E, HAS-R, HAS-ER HAS-HCR	M8x80	M10x90	M12x110	M16x125	M20x170	M24x210	M30x270
Anchor embedment depth [mm]	80	90	110	125	170	210	270

a) M30 design please use anchor design software PROFIS anchor.

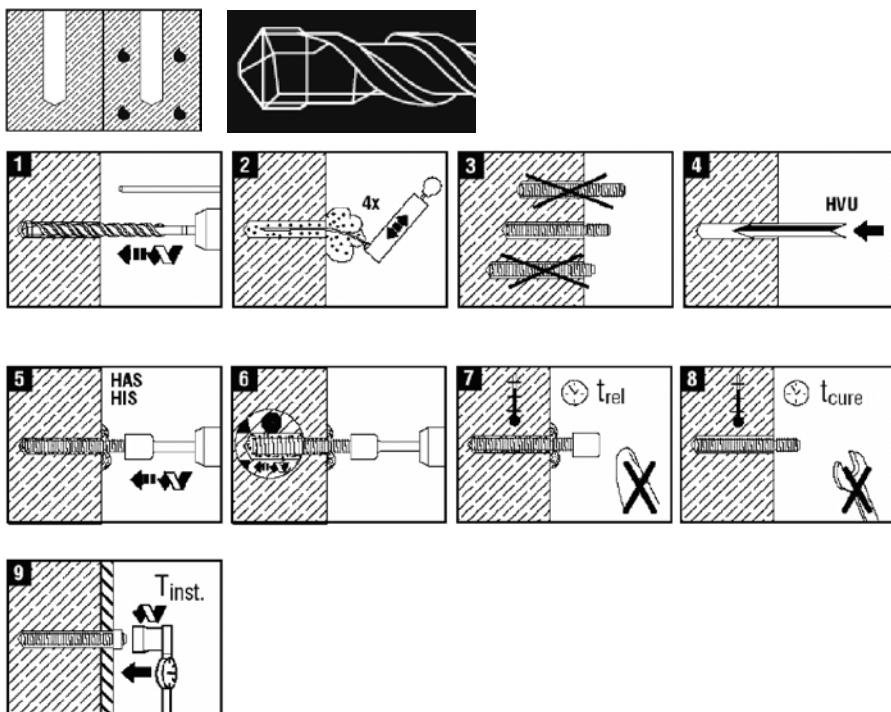
Setting

Installation equipment

Anchor size	M8	M10	M12	M16	M20	M24	M30
Rotary hammer			TE 2 – TE 30				TE 40 – TE 70
Other tools				compressed air gun or blow out pump, set of cleaning brushes, dispenser			

Setting instructions

Dry and water-saturated concrete, hammer drilling



For detailed information on installation see instruction for use given with the package of the product.

Curing time for general conditions

Data according ETA-05/0255, issue 2011-06-23	
Temperature of the base material	Curing time before anchor can be fully loaded t_{cure}
20 °C to 40 °C	20 min
10 °C to 19 °C	30 min
0 °C to 9 °C	1 h
-5 °C to - 1 °C	5 h

Setting details

		Data according ETA-05/0255, issue 2011-06-23						
Anchor size		M8	M10	M12	M16	M20	M24	M30
Nominal diameter of drill bit	d_0 [mm]	10	12	14	18	24	28	35
Effective anchorage and drill hole depth	$h_{ef,min}$ [mm]	80	90	110	125	170	210	270
Max. fixture thickness	$t_{fix,max}$ [mm]	14	21	28	38	48	54	70
Diameter of clearance hole in the fixture	d_f [mm]	9	12	14	18	22	26	33
Minimum spacing	s_{min} [mm]	40	45	55	65	90	120	135
Minimum edge distance	c_{min} [mm]	40	45	55	65	90	120	135
Torque moment ^{a)}	t_{max} [Nm]	10	20	40	80	150	200	300

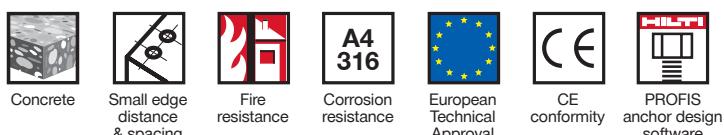
a) This is the maximum recommended torque moment to avoid splitting failure during installation for anchors with minimum spacing and/or edge distance.

HILTI

**HVU with HAS-E rod
adhesive anchor**

HVU with HIS-(R)N adhesive anchor

Mortar System	Benefits
 	<ul style="list-style-type: none"> ■ suitable for non-cracked concrete C 20/25 to C 50/60 ■ high loading capacity ■ suitable for dry and water saturated concrete



Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval ^{a)}	DIBt, Berlin	ETA-05/0255 / 2011-06-23
Fire test report	IBMB, Braunschweig	UB-3333/0891-1 / 2004-03-26
Assessment report (fire)	warringtonfire	WF 166402 / 2007-10-26

a) All data given in this section according ETA-05/0255, issue 2011-06-23

Service temperature range

Hilti HVU adhesive may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +40 °C	+24 °C	+40 °C
Temperature range II	-40 °C to +80 °C	+50 °C	+80 °C
Temperature range III	-40 °C to +120 °C	+72 °C	+120 °C

Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

Design process for typical anchor layouts

The design values in the tables are obtained from the design method according to ETAG 001, Annex C and Hilti simplified design method. Design resistance according to data given in ETA-05/0255, issue 2011-06-23.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

The values are valid for the anchor configuration.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

STEP 1: TENSION LOADING

The design tensile resistance N_{Rd} is the lower of:

- **Combined pull-out and concrete cone resistance**

$$N_{Rd,p} = f_{B,p} \cdot N^*_{Rd,p}$$

$N^*_{Rd,p}$ is obtained from the relevant design tables

$f_{B,p}$ influence of concrete strength on combined pull-out and concrete cone resistance

Concrete Strengths $f'_{c,cyl}$ (MPa)	20	25	32	40	50
$f_{B,p}$	0.95	0.97	1.00	1.02	1.04

- **Concrete cone or concrete splitting resistance**

$$N_{Rd,c} = f_B \cdot N^*_{Rd,c}$$

$N^*_{Rd,c}$ is obtained from the relevant design tables

f_B influence of concrete strength on concrete cone resistance

Concrete Strengths $f'_{c,cyl}$ (MPa)	20	25	32	40	50
f_B	0.79	0.87	1.00	1.11	1.22

- **Design steel resistance (tension) $N_{Rd,s}$**

Anchor size		M8	M10	M12	M16	M20
NRd,s	HIS-N [kN]	17.5	30.7	44.7	80.3	74.1
	HIS-RN [kN]	13.9	21.9	31.6	58.8	69.2
	Bolt Grade 5.8 [kN]	12.0	19.3	28.0	52.7	82.0
	Bolt Grade 8.8 [kN]	19.3	30.7	44.7	84.0	130.7
	Bolt Grade A 4-70 / 316 [kN]	13.9	21.9	31.6	58.8	92.0

Note: Designer needs to check the bolt tensile resistance.

$$N_{Rd} = \min \{ N_{Rd,p}, N_{Rd,c}, N_{Rd,s} \}$$

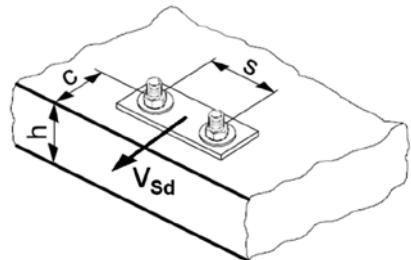
CHECK $N_{Rd} \geq N_{Sd}$

STEP 2: SHEAR LOADING

The design shear resistance V_{Rd} is the lower of:

■ Design Concrete Edge Resistance

$$V_{Rd,c} = f_B \cdot V^{*}_{Rd,c}$$



$V^{*}_{Rd,c}$ is obtained from the relevant design table

f_B influence of concrete strength

Concrete Strengths f'_c, cyl (MPa)	20	25	32	40	50
f_B	0.79	0.87	1.00	1.11	1.22

Shear load acting parallel to edge:

These tables are for a single free edge only

2 anchors:

For shear loads acting parallel to this edge, the concrete resistance $V^{*}_{Rd,c}$ can be multiplied by the factor = 2.5

4 anchors:

For shear loads acting parallel to the edge - the anchor row closest to the edge is checked to resist half the total design load. To obtain the concrete resistance use the corresponding 2 anchor configuration $V^{*}_{Rd,c}$ and multiply by the factor = 2.5

■ Design steel resistance (shear): $V_{Rd,s}$

Anchor size	M8	M10	M12	M16	M20
$V_{Rd,s}$	HIS-N [kN]	10.4	18.4	26.0	39.3
	HIS-RN [kN]	8.3	12.8	19.2	35.3
Bolt Grade 5.8 [kN]	7.2	12.0	16.8	31.2	48.8
Bolt Grade 8.8 [kN]	12.0	18.4	27.2	50.4	78.4
Bolt Grade A 4-70 / 316 [kN]	8.3	12.8	19.2	35.3	55.1

Note: Designer needs to check the bolt shear resistance.

$$V_{Rd} = \min \{ V_{Rd,c}, V_{Rd,s} \}$$

CHECK $V_{Rd} \geq V_{sd}$

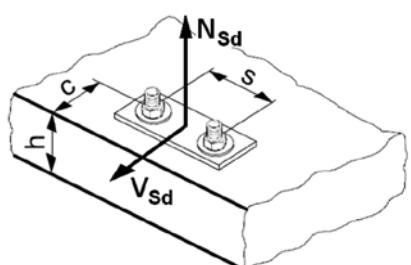
STEP 3: COMBINED TENSION AND SHEAR LOADING

The following equations must be satisfied:

$$N_{sd}/N_{Rd} + V_{sd}/V_{Rd} \leq 1.2$$

and

$$N_{sd}/N_{Rd} \leq 1, V_{sd}/V_{Rd} \leq 1$$



Precalculated table values – design resistance values

General:

The following tables provide the total ultimate limit state design resistance for the configurations.
All tables are based upon:

- correct setting (See setting instruction)
- non-cracked concrete – $f_{c,cyl} = 32 \text{ MPa}$
- temperature range I (see service temperature range)
- base material thickness, as specified in the table
- One typical embedment depth, as specified in the tables

Basic loading data (for a single anchor) – no edge distance and spacing influence

Embedment depth and base material thickness for the basic loading data

Anchor size	M8	M10	M12	M16	M20
Embedment depth [mm]	90	110	125	170	205
Base material thickness [mm]	120	150	170	230	270

Design resistance [kN] – uncracked concrete, 32 Mpa

Anchor size	M8	M10	M12	M16	M20
Non-cracked concrete					
Tensile	Pull-out $N^*_{Rd,p}$	19.0	30.4	45.7	72.3
	Concrete $N^*_{Rd,c}$	36.4	49.1	59.5	94.4
Shear	$V_{Rd,s}$	Steel governed refer $V_{Rd,s}$ table			

Basic loading data (for a single anchor) – with minimum edge distance

Design resistance [kN] - uncracked concrete, 32 Mpa

Anchor size	M8	M10	M12	M16	M20
Min. edge distance c_{min} [mm]	40	45	60	80	125
Min Base thickness h_{min} [mm]	120	150	170	230	270
Tensile N_{Rd}					
	Pull-out $N^*_{Rd,p}$ [kN]	10.1	15.3	24.0	38.2
	Concrete $N^*_{Rd,c}$ [kN]	16.6	22.1	27.9	44.0
Shear V_{Rd}					
	Shear $V^*_{Rd,c}$ (without lever arm)	5.3	7.0	10.7	17.4
					32.1

ANCHOR M16	Edge C (mm)														
	80			100			150			200			250		
	tension		shear	tension											
spacing s1 (mm)	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}
80	48.0	48.7	23.3	52.6	52.0	28.9	64.6	60.6	44.3	77.9	69.7	53.8	90.8	79.4	62.7
100	49.4	49.9	24.7	54.1	53.3	30.4	66.5	62.0	46.0	80.1	71.4	55.3	93.5	81.4	64.2
150	52.9	52.9	28.4	57.9	56.5	34.2	71.2	65.8	50.1	85.8	75.7	59.3	100.1	86.2	67.9
200	56.4	55.9	32.0	61.8	59.7	38.0	75.9	69.5	54.3	91.5	80.0	63.3	106.7	91.1	71.7
250	59.9	58.9	34.9	65.6	62.9	41.8	80.7	73.2	58.5	97.1	84.2	67.2	113.3	96.0	75.5
300	63.4	61.9	34.9	69.4	66.0	45.6	85.3	76.9	62.7	102.8	88.5	71.2	119.9	108.0	79.3

ANCHOR M20	Edge C (mm)														
	125			150			200			250			300		
	tension		shear												
spacing s1 (mm)	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}
125	81.7	70.8	42.7	89.4	75.3	51.2	105.9	84.9	65.7	123.7	95.0	75.3	136.0	105.6	84.9
150	84.3	72.5	44.9	92.3	77.2	53.4	109.3	86.9	68.0	127.7	97.3	77.5	140.4	108.1	87.0
200	89.6	75.9	49.2	98.1	80.8	57.8	116.2	91.0	72.5	135.7	101.8	81.8	149.3	113.2	91.1
250	94.9	79.3	53.4	103.9	84.4	62.3	123.1	95.1	77.0	143.7	106.4	86.1	158.1	118.3	95.3
300	100.2	82.7	57.7	109.7	88.0	66.7	129.9	99.2	81.5	151.7	111.0	90.4	166.9	123.3	99.4
350	105.4	86.1	62.0	115.5	91.7	71.2	136.8	103.3	86.1	159.7	115.5	94.7	175.7	128.4	103.5

Shear design: The concrete edge resistance value in this table uses all 4 anchors in shear. You will need to ensure the gap between anchor and the plate is filled. This can be achieved using the Hilti Dynamic Set. (Refer page 41 for further details)

The concrete edge resistance values have been obtained by taking the lesser of:

1. First row resistance multiplied by number of rows and
2. The concrete edge resistance of the furthest row.

ANCHOR M16	Edge C (mm)														
	80			100			150			200			250		
	tension		shear												
s1= s2 (mm)	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}
80	66.1	57.5	46.5	71.5	60.9	50.2	85.9	69.9	59.1	101.5	79.5	67.9	116.7	89.7	76.7
100	70.9	61.1	49.5	76.5	64.7	55.3	91.4	74.0	64.2	107.6	84.0	72.9	123.4	94.5	81.6
150	83.5	70.6	56.7	89.7	74.6	67.9	106.1	84.8	76.5	123.9	95.7	85.1	141.2	107.2	93.6
200	97.1	80.9	64.0	103.9	85.1	76.0	121.9	96.3	88.6	141.2	108.1	97.1	160.1	120.6	105.5
250	111.6	91.8	69.8	119.0	96.4	83.6	138.6	108.5	100.5	159.6	121.3	108.9	180.0	134.8	117.1
300	126.8	103.3	69.8	134.9	108.4	91.2	156.1	121.4	112.3	178.9	135.2	120.5	201.0	149.8	128.7

ANCHOR M20	Edge C (mm)														
	125			150			200			250			300		
	tension		shear												
s1= s2 (mm)	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}	N* _{Rd,p}	N* _{Rd,c}	V* _{Rrd,c}
125	113.2	86.0	75.3	122.3	90.9	80.1	141.5	101.1	89.7	162.1	111.8	99.2	176.3	123.0	108.6
150	121.8	91.1	82.2	131.3	96.2	87.0	151.4	106.8	96.4	172.9	117.9	105.8	187.7	129.5	115.2
200	139.9	101.9	95.8	150.3	107.4	100.4	172.2	118.7	109.7	195.5	130.6	119.0	211.6	143.1	128.2
250	159.1	113.3	106.9	170.4	119.2	113.7	194.2	131.3	122.8	219.5	144.0	132.0	237.0	157.3	141.1
300	179.5	125.3	115.4	191.7	131.5	126.7	217.4	144.5	135.7	244.8	158.1	144.8	263.6	172.2	153.8
350	200.9	137.8	124.0	214.2	144.5	139.5	241.9	158.3	148.5	271.3	172.7	157.5	291.6	187.8	166.4

Materials

Mechanical properties of HIS-(R)N

Anchor size			M8x90	M10x110	M12x125	M16x170	M20x205
Nominal tensile strength f_{uk}	HIS-N	[N/mm ²]	490	490	460	460	460
	Screw 8.8	[N/mm ²]	800	800	800	800	800
	HIS-RN	[N/mm ²]	700	700	700	700	700
	Screw A4-70	[N/mm ²]	700	700	700	700	700
Yield strength f_{yk}	HIS-N	[N/mm ²]	410	410	375	375	375
	Screw 8.8	[N/mm ²]	640	640	640	640	640
	HIS-RN	[N/mm ²]	350	350	350	350	350
	Screw A4-70	[N/mm ²]	450	450	450	450	450
Stressed cross-section A_s	HIS-(R)N	[mm ²]	51.5	108.0	169.1	256.1	237.6
	Screw	[mm ²]	36.6	58	84.3	157	245
Section modulus Z	HIS-(R)N	[mm ³]	145	430	840	1595	1543
	Screw	[mm ³]	31.2	62.3	109	277	541

Material quality

Part	Material
Internal threaded sleeve ^{a)} HIS-N	C-steel 1.0718 Steel galvanized $\geq 5\mu\text{m}$
Internal threaded sleeve ^{b)} HIS-RN	Stainless steel 1.4401; 1.4571

a) related fastening screw: strength class 8.8 EN ISO 898-1, $A_s > 8\%$ Ductile steel galvanized $\geq 5\mu\text{m}$

b) related fastening screw: strength class 70 EN ISO 3506-1, $A_s > 8\%$ Ductile stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362

Anchor dimensions

Anchor size	M8	M10	M12	M16	M20
Internal sleeve HIS-(R)N	M8x90	M10x110	M12x125	M16x170	M20x205
Anchor embedment depth [mm]	90	110	125	170	205

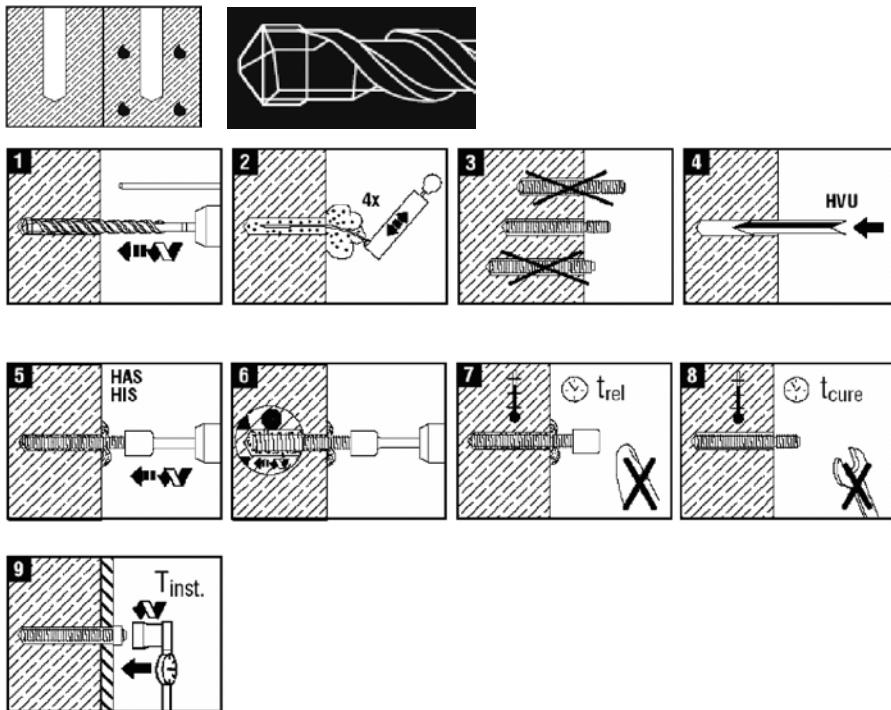
Setting

Installation equipment

Anchor size	M8	M10	M12	M16	M20
Rotary hammer	TE 2 – TE 30			TE 40 – TE 70	
Other tools					blow out pump or compressed air gun, setting tools

Setting instructions

Dry and water-saturated concrete, hammer drilling



For detailed information on installation see instruction for use given with the package of the product.

Curing time for general conditions

Data according ETA-05/0255, issue 2011-06-23	
Temperature of the base material	Curing time before anchor can be fully loaded t_{cure}
20 °C to 40 °C	20 min
10 °C to 19 °C	30 min
0 °C to 9 °C	1 h
-5 °C to - 1 °C	5 h

Setting details

		Data according ETA-05/0255, issue 2011-06-23				
Anchor size	Sleeve HIS-(R)N foil capsule	M8x90 M10x90	M10x110 M12x110	M12x125 M16x125	M16x170 M20x170	M20x205 M24x210
Nominal diameter of drill bit	d_0 [mm]	14	18	22	28	32
Diameter of element	d [mm]	12.5	16.5	20.5	25.4	27.6
Effective anchorage and drill hole depth	$h_{ef,min}$ [mm]	90	110	125	170	205
Diameter of clearance hole in the fixture	d_f [mm]	9	12	14	18	22
Thread engagement length; min - max	h_s [mm]	8-20	10-25	12-30	16-40	20-50
Minimum spacing	s_{min} [mm]	40	45	60	80	125
Minimum edge distance	c_{min} [mm]	40	45	60	80	125
Torque moment ^{a)}	t_{max} [Nm]	10	20	40	80	150

a) This is the maximum recommended torque moment to avoid splitting failure during installation for anchors with minimum spacing and/or edge distance.